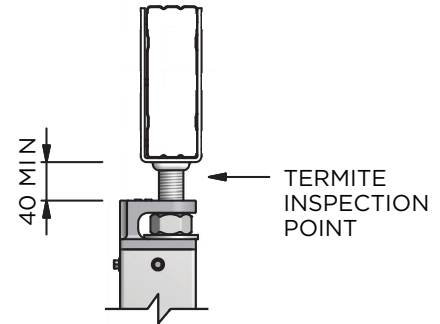


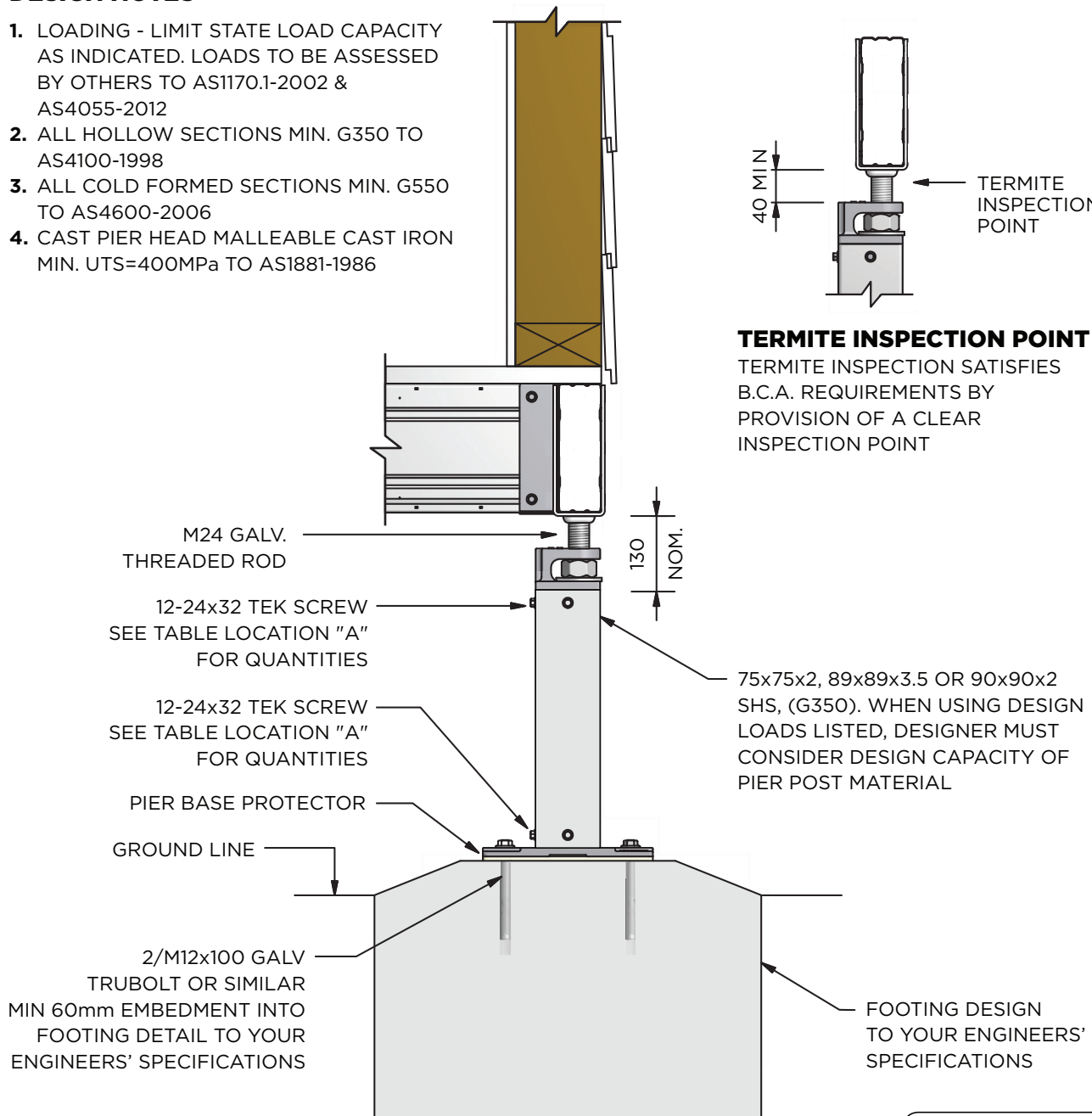
DESIGN NOTES

1. LOADING - LIMIT STATE LOAD CAPACITY AS INDICATED. LOADS TO BE ASSESSED BY OTHERS TO AS1170.1-2002 & AS4055-2012
2. ALL HOLLOW SECTIONS MIN. G350 TO AS4100-1998
3. ALL COLD FORMED SECTIONS MIN. G550 TO AS4600-2006
4. CAST PIER HEAD MALLEABLE CAST IRON MIN. UTS=400MPa TO AS1881-1986



TERMITE INSPECTION POINT

TERMITE INSPECTION SATISFIES B.C.A. REQUIREMENTS BY PROVISION OF A CLEAR INSPECTION POINT



DESIGN LOAD CAPACITIES

LIMIT STATE VERTICAL DOWNWARD LOAD CAPACITY 54kN
LIMIT STATE MAXIMUM UPLIFT LOAD CAPACITY 27kN

TEK SCREW QTY LOCATION "A"	MAX UPLIFT CAPACITY kN
6	27.0
4	19.2
4	16.2
3	10.8
2	9.6

Structural Design Certification By:

DESIGN. PLAN. MANAGE
ACN 088 342 645
136 Darling Street, Dubbo NSW 2830

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BE(Hons)ME MIEAust CPEng
NPER 472690, RPEQ 5264,
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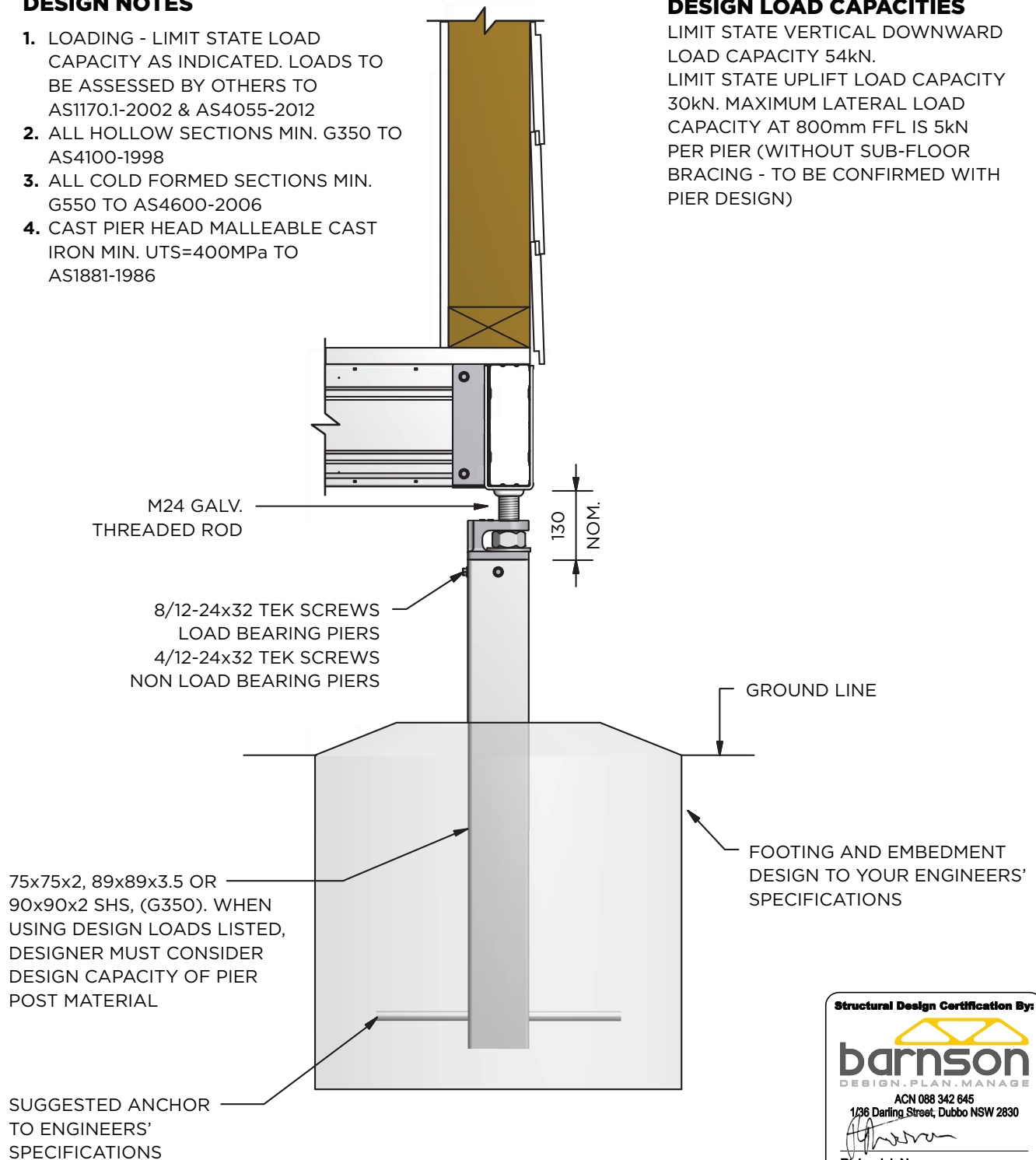
Date: 18-09-2014
Reference No: 20727

DESIGN NOTES

1. LOADING - LIMIT STATE LOAD CAPACITY AS INDICATED. LOADS TO BE ASSESSED BY OTHERS TO AS1170.1-2002 & AS4055-2012
2. ALL HOLLOW SECTIONS MIN. G350 TO AS4100-1998
3. ALL COLD FORMED SECTIONS MIN. G550 TO AS4600-2006
4. CAST PIER HEAD MALLEABLE CAST IRON MIN. UTS=400MPa TO AS1881-1986

DESIGN LOAD CAPACITIES

LIMIT STATE VERTICAL DOWNWARD LOAD CAPACITY 54kN.
LIMIT STATE UPLIFT LOAD CAPACITY 30kN. MAXIMUM LATERAL LOAD CAPACITY AT 800mm FFL IS 5kN PER PIER (WITHOUT SUB-FLOOR BRACING - TO BE CONFIRMED WITH PIER DESIGN)



Structural Design Certification By:

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Date: 18-09-2014
Reference No: 20727

SPANTEC <small>SPANTEC SYSTEMS Pty Ltd ABN 56 053 584 384</small>	<small>17 Drapers Road Braemar NSW 2575 PO Box 81 Mittagong NSW 2575 Australia Phone 02 4860 1000 Fax 02 4872 1616 www.spantec.com.au</small>	<small>DATE DRAWN</small> 03/04/15	<small>DWG FILE</small> 	<small>JOB DETAILS</small> EZIPIER CYCLONIC WIND CONDITIONS	<small>DESIGNED</small> GMC	<small>JOB REFERENCE</small> P04-02
	<small>SCALE</small> NTS	<small>VIEW NAME</small> 	<small>REVISION</small> 			

TABLE B: EZIPIER UPLIFT CAPACITY

LOCATION	A	B
WEB 0.8BMT LOAD (kN)	18.0	24.9
WEB 1.0BMT LOAD (kN)	24.9	35.8
FASTENER QTY	12 x 14g TEKS	12 x 14g TEKS
FASTENER QTY	4 x 12g TEKS	6 x 12g TEKS
	8 x 12g TEKS	8 x 12g TEKS
	12 x 14g TEKS+1 x M10 BOLT	10 x 12g TEKS

TABLE A: EZIPIER DOWNWARD CAPACITY
(MAX. FFL 2700mm)

PIER HEAD	PIER SHS SIZE (mm)	MAX. DOWN LOAD (kN)
75LPH	75x75x2.0	45
90LPH	90x90x2.0	55
89LPH	89x89x3.5	110

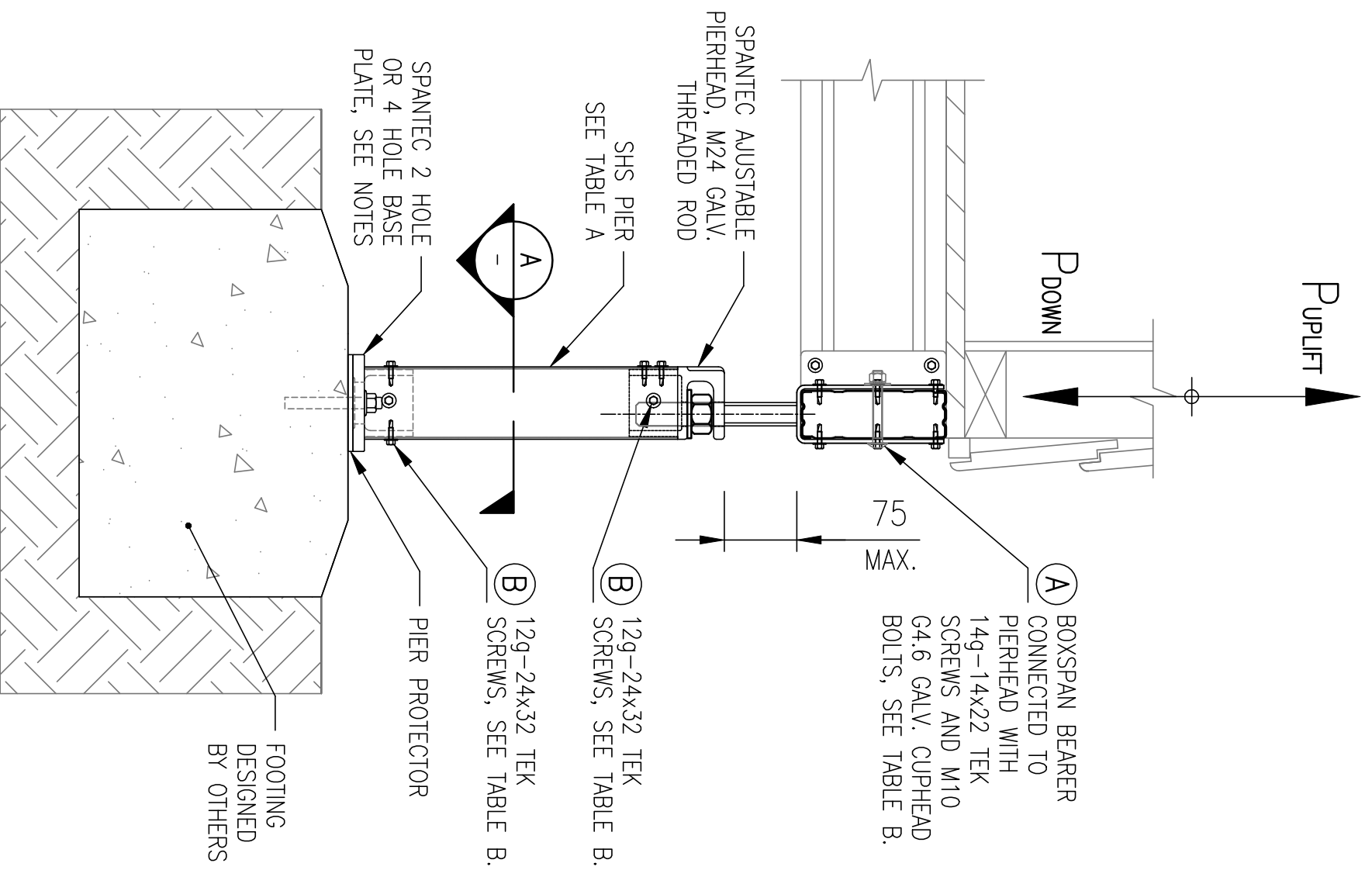
EZIPIER DOWNWARD CAPACITY NOTES

THE CAPACITY OF PIERHEAD IS BASED ON THE STRENGTH OF THE WHOLE PIER ASSEMBLY.
THE ULTIMATE DOWNWARD LOAD CAPACITY OF THE PIER IS BASED ON A MAXIMUM FFL OF 2700mm, FOR FLOOR HEIGHTS ABOVE 2700mm THE PIER CAPACITY MUST BE CHECKED BY YOUR LOCAL ENGINEER.
EZIPIER CAN BE SUPPLIED WITH A 2 OR 4 HOLE BASE PLATE.
SHS MIN. STEEL GRADE 350MPa TO AS1163. THE CAPACITIES IN THE TABLES ARE CALCULATED USING AUSTRALIAN LOADING CODE AS1170 AND AS4600.

BOXSPAN LEGEND:
WEB 0.8BMT = B100-16, B150-16, B200-16
WEB 1.0BMT = B150-20, B200-20, B250-20

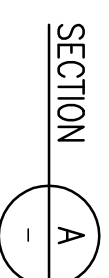
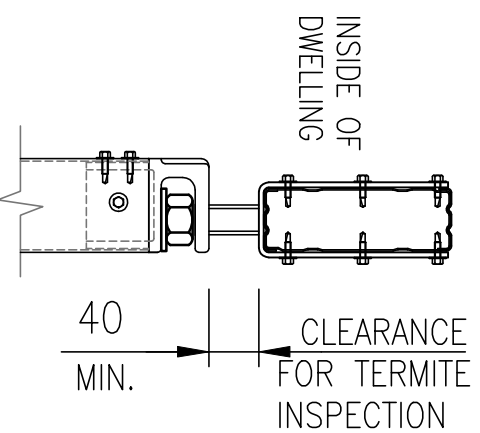
GENERAL NOTES:

- IN THIS DESIGN IT IS ASSUMED THE FLOOR SUPPORTED BY THE PIERS IS FULLY BRACED AND THE LOADS SUPPORTED ARE DEAD LOADS, LIVE LOADS AND WIND UPLIFT ONLY.
- THIS PIERHEAD IS TO BE USED IN CONJUNCTION WITH THE FLOOR BEAM MANUFACTURERS RECOMMENDED BLOCKING REQUIREMENTS TO RESIST RACKING LOADS.
- THE NOMINAL CONNECTION SHOWN IS THE MINIMUM CONNECTION THAT SHOULD BE USED. THE ENGINEER SHOULD CHECK AND DESIGN A SUITABLE CONNECTION FOR UPLIFT BASED ON REAL LOADS.
- BASE PLATE MUST BE SELECTED TO SUIT THE APPLIED LOADS, SEE DRAWING P14 FOR THE 2 AND 4 HOLE BASE PLATE CAPACITIES OR VISIT OUR WEBSITE www.spantec.com.au



FOOTING DESIGNED BY OTHERS

EZIPIER END ELEVATION



NOMINAL CONNECTION
2/M12x100 LG GALV. WEDGE ANCHORS IN 110mm DEEP HOLE WITH 60mm MIN. EMBEDMENT (AFTER TIGHTENING) IN N25 CONCRETE.

TERMITE INSPECTION POINT AND PIER HEAD PREFERRED ORIENTATION

TERMITE INSPECTION SATISFIES NCC REQUIREMENTS BY PROVISION OF A CLEAR INSPECTION POINT.

NOTE:
BASE PLATE ORIENTATION IS PARALLEL TO BEARER.

Structural Design Certification By:

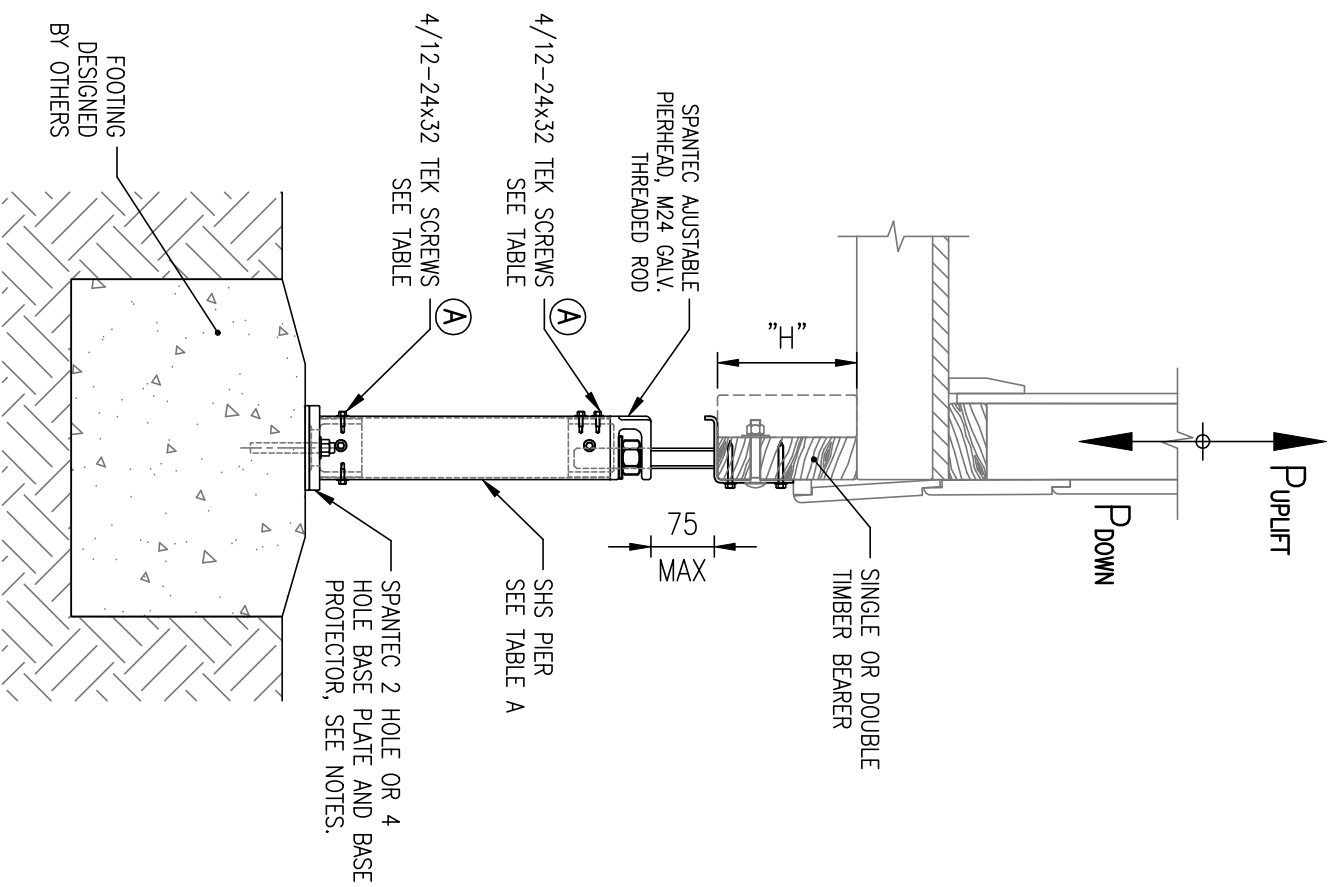
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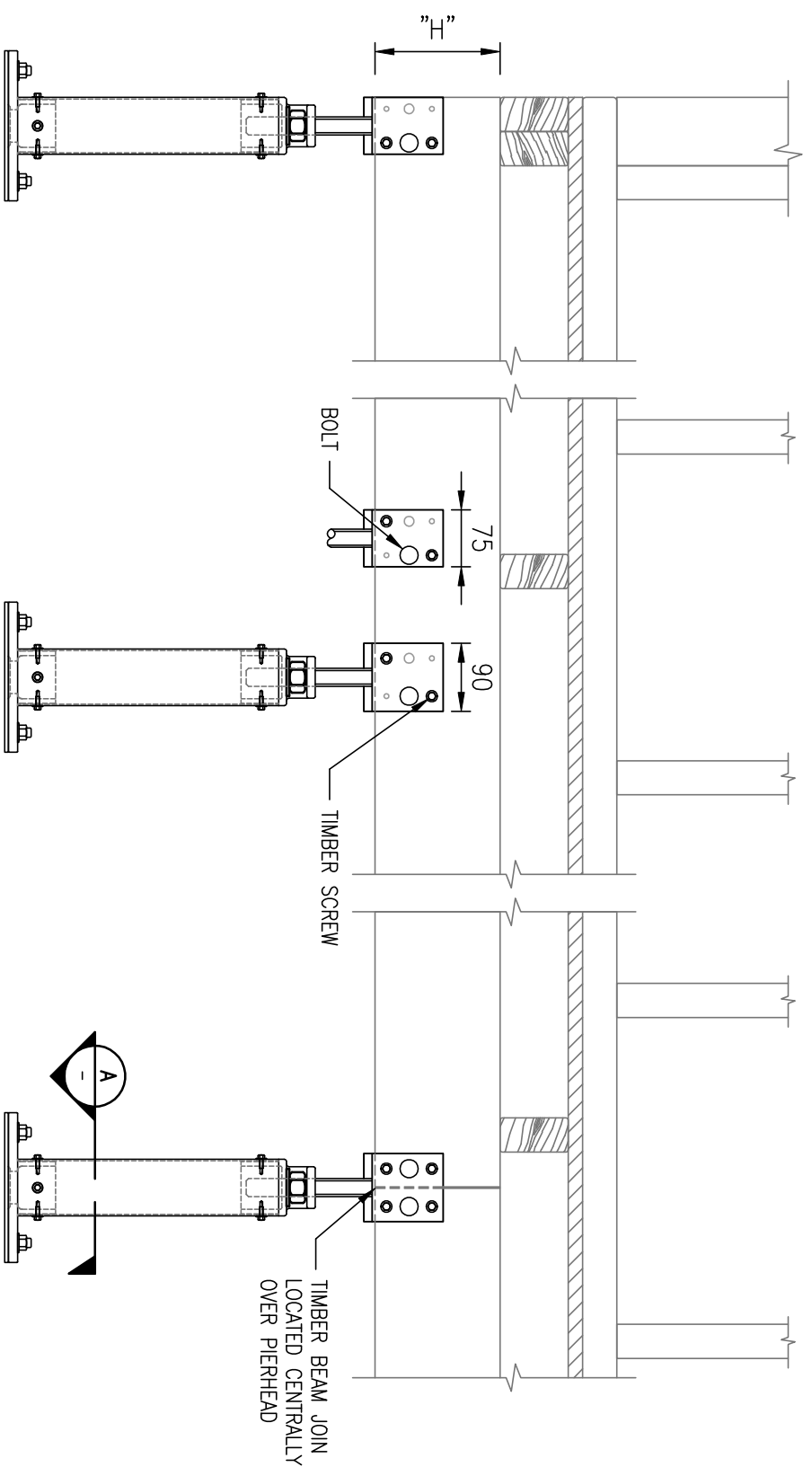
PETER J. BARRECA
Civil & Structural Engineer
Engineers Australia Membership N° 353640
NER National Engineering Register
CEng Chartered Professional Engineer
Qld PRECO N° 7678
VIC RBP Civil Engineer VBA N° EC2407
TAS Civil Engineer CC5469A

REV.	DESCRIPTION	DRN.	DATE
-	-	-	-

DESCRIPTION	DRAWING NO.	REVISION
EZIPIER ADJUSTABLE "U" PIERHEAD ASSEMBLY BOXSPAN CONNECTION DETAILS	P04-01	-
SCALE @ A3 NTS	DRAWN AP	DATE DRAWN 16/11/20



EZIPIER END ELEVATION



**END SPAN CONNECTION
75mm OR 90mm LONG PIERHEAD (75/90LPH)**

PIER HEAD/BEARER END CONNECTION

- a. FOR BEARERS WHERE $H \leq 150\text{mm}$
USE 1/M10 G4.6 GALVANISED CUP HEAD BOLT.
- b. FOR BEARERS WHERE $H > 150\text{mm}$
USE 1/M10 G4.6 GALVANISED CUP HEAD BOLT PLUS 2/14g GALVANISED SCREWS 50 MIN. LENGTH.

**INTERNAL SPAN CONNECTION
75 OR 90mm LONG PIERHEAD (75/90LPH)**

PIER HEAD/BEARER MID SPAN CONNECTION

- a. FOR BEARERS WHERE $H \leq 150\text{mm}$
USE 1/M10 G4.6 GALVANISED CUP HEAD BOLT.
- b. FOR BEARERS WHERE $H > 150\text{mm}$
USE 1/M10 G4.6 GALVANISED CUP HEAD BOLT PLUS 2/14g GALVANISED SCREWS 50 MIN. LENGTH.

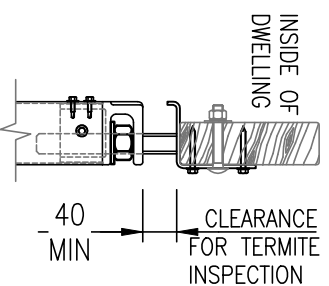
**JOIN CONNECTION
90mm LONG PIERHEAD (90LPH)**

PIER HEAD/BEARER JOIN CONNECTION

- USE 2/M10 G4.6 GALVANISED CUP HEAD BOLT PLUS 4/14g GALV. SCREWS x 50 MIN. LENGTH

- NOTES:**
- IT IS ASSUMED THE FLOOR SUPPORTED BY THE PIERS IS FULLY BRACED AND THE LOADS SUPPORTED ARE DEAD LOADS, LIVE LOADS AND WIND UPLIFT ONLY.
 - THIS PIERHEAD IS TO BE USED IN CONJUNCTION WITH THE FLOOR BEAM MANUFACTURERS RECOMMENDED BLOCKING REQUIREMENTS TO RESIST RACKING LOADS.
 - THE FASTENERS SHOWN FOR THE BEAM/PIERHEAD CONNECTION ARE FOR A STRUCTURAL TIMBER BEAM AND IS THE MINIMUM CONNECTION RECOMMENDED FOR NOMINAL LOADS. THE DESIGNER HAS TO CHECK THE TIMBER BEARER/PIERHEAD CONNECTED TO SUIT THE ACTUAL LOADS.
 - PIERHEAD CAN BE ORIENTED WITH LIP POINTING IN OR OUT, PREFERRED ORIENTATION AS SHOWN.
 - THE PIER/FOOTING CONNECTION DETAIL IS VALID FOR SPANTEC PRODUCTS ONLY. IF OTHER PRODUCTS ARE USED THE LOAD CAPACITIES ARE NOT GUARANTEED. SEEK ADVICE FROM A LOCAL ENGINEER FOR YOUR SPECIFIC ARRANGEMENT.
 - BASE PLATE MUST BE SELECTED TO SUIT THE APPLIED LOADS, SEE DRAWING P14 FOR THE 2 AND 4 HOLE BASE PLATE CAPACITIES.

TERMITE INSPECTION POINT AND PIER HEAD PREFERRED ORIENTATION
TERMITE INSPECTION SATISFIES NCC REQUIREMENTS BY PROVISION OF A CLEAR INSPECTION POINT.



NOMINAL CONNECTION
2/M12x100 LG GALV. WEDGE ANCHORS IN 110mm DEEP HOLE WITH 60mm MIN. EMBEDMENT (AFTER TIGHTENING) IN N25 CONCRETE.

SPANTEC 2 HOLE OR 4 HOLE BASE PLATE

NOTE:
BASE PLATE ORIENTATION IS PARALLEL TO BEARER.



EZIPIER DOWNWARD CAPACITY (SEE TABLE A)
THE CAPACITY OF PIERHEAD IS BASED ON THE STRENGTH OF THE WHOLE PIER ASSEMBLY. THE ULTIMATE DOWNWARD LOAD CAPACITY OF THE PIER IS BASED ON A MAXIMUM FFL OF 2700mm, FOR FLOOR HEIGHTS ABOVE 2700mm THE PIER CAPACITY MUST BE CHECKED BY YOUR LOCAL ENGINEER. EZIPIER CAN BE SUPPLIED WITH A 2 OR 4 HOLE BASE PLATE.

TABLE A: EZIPIER DOWNWARD CAPACITY
(MAX. FFL 2700mm)

PIER HEAD	PIER SHS SIZE (mm)	MAX. DOWN LOAD (kN)
75LPH	75x75x2.0	45
90LPH	90x90x2.0	55
89LPH	89x89x3.5	110

EZIPIER UPLIFT CAPACITY (SEE TABLE B)
THE MIN. ULTIMATE UPLIFT LOAD CAPACITY FOR THE CONNECTION AS SHOWN IS 17.3kN (EXCLUDES THE BEAM/PIERHEAD CONNECTION). SEE TABLE IF HIGHER VALUES ARE REQUIRED. THE UPLIFT, IF ANY, ON THE BEAM/PIER HEAD CONNECTION MUST BE CALCULATED BY YOUR LOCAL ENGINEER. SEE DRAWING P14 FOR THE UPLIFT CAPACITY OF 2 HOLE AND 4 HOLE BASEPLATE OR VISIT OUR WEBSITE www.spantec.com.au

TABLE B: EZIPIER UPLIFT CAPACITY

TEK "A" QTY	MAX. UP LOAD (kN)
4	17.3
6	26.0
8	34.6

NOTE:
THE UPLIFT CAPACITIES IN TABLE ABOVE ARE SUITABLE FOR 75x2, 90x2 AND 89x3.5 SHS. SHS MIN. STEEL GRADE 350MPa TO AS1163. THE CAPACITIES IN THE TABLES ARE CALCULATED USING AUSTRALIAN LOADING CODE AS1170 AND AS4600.

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NER National Engineering Register
CPEng Chartered Professional Engineer
Qld Reg'd N° 7078
Vic Reg'd Civil Engineer VBA N° EC24807
Tas Civil Engineer CD456A

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REV.	DESCRIPTION	DRN.	DATE
A	TERMITE INSPECTION ADDED.	AP	14-4-20
B	VERTICAL UP AND DOWNWARD CAPACITY ADDED.	AP	28-10-20

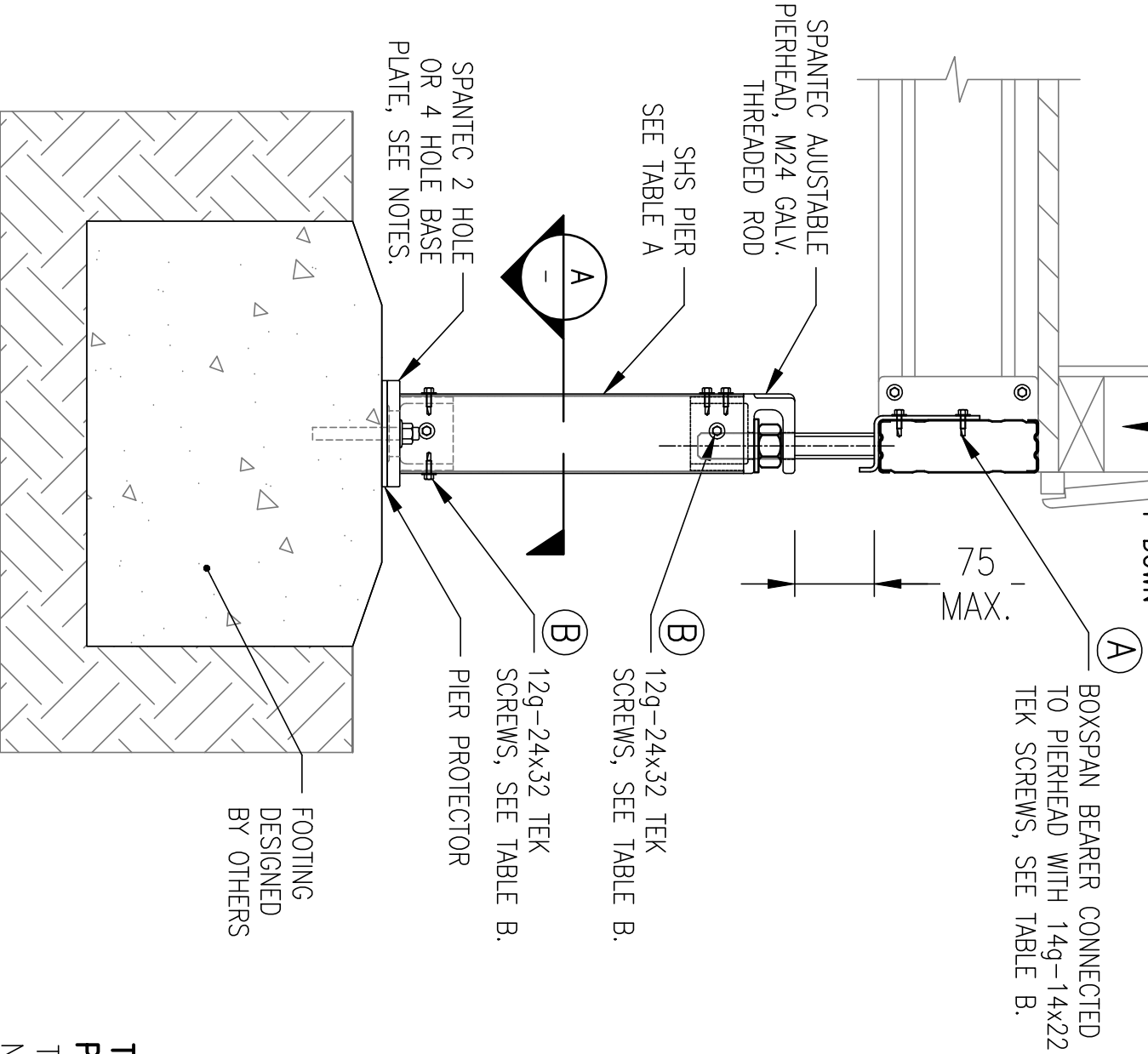
DESCRIPTION
EZIPIER ADJUSTABLE "L" PIERHEAD TIMBER CONNECTION DETAILS

DRAWING NO.	REVISION
P06	B

SCALE @ A3	DRAWN	DATE DRAWN
NTS	AP	5/08/19

P_{UPLIFT}

P_{DOWN}



EZUPIER END ELEVATION

TABLE B: EZUPIER UPLIFT CAPACITY

TEK "A" QTY	TEK "B" QTY	WEB 0.8BMT LOAD (kN)	WEB 1.0BMT LOAD (kN)
6	4	12.7	17.7

BOXSPAN LEGEND:
 WEB 0.8BMT = B100-16, B150-16, B200-16
 WEB 1.0BMT = B150-20, B200-20, B250-20

GENERAL NOTES:

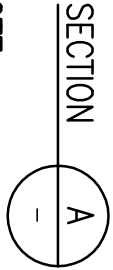
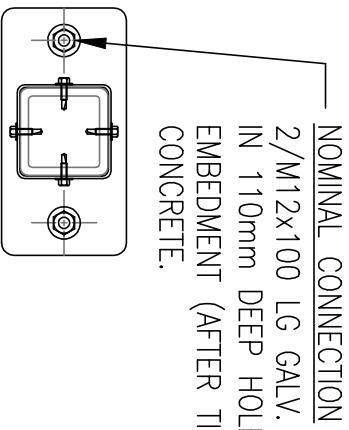
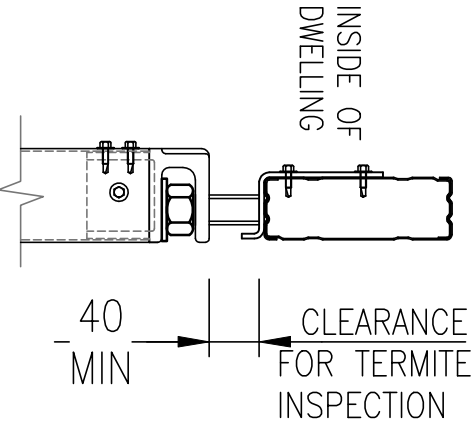
- IT IS ASSUMED THE FLOOR SUPPORTED BY THE PIERS IS FULLY BRACED AND THE LOADS SUPPORTED ARE DEAD LOADS, LIVE LOADS AND WIND UPLIFT ONLY.
- THIS PIERHEAD IS TO BE USED IN CONJUNCTION WITH THE FLOOR BEAM MANUFACTURERS RECOMMENDED BLOCKING REQUIREMENTS TO RESIST RACKING LOADS.
- THE NOMINAL CONNECTION SHOWN IS THE MINIMUM CONNECTION THAT SHOULD BE USED. THE ENGINEER SHOULD CHECK AND DESIGN A SUITABLE CONNECTION FOR UPLIFT BASED ON REAL LOADS.
- BASE PLATE MUST BE SELECTED TO SUIT THE APPLIED LOADS, SEE DRAWING P14 FOR THE 2 AND 4 HOLE BASE PLATE CAPACITIES OR VISIT OUR WEBSITE www.spantec.com.au

TABLE A: EZUPIER DOWNWARD CAPACITY
(MAX. FFL 2700mm)

PIER HEAD	PIER SHS SIZE (mm)	MAX. DOWN LOAD (kN)
75LPH	75x75x2.0	45
90LPH	90x90x2.0	55
89LPH	89x89x3.5	110

EZUPIER DOWNWARD CAPACITY NOTES

THE CAPACITY OF PIERHEAD IS BASED ON THE STRENGTH OF THE WHOLE PIER ASSEMBLY. THE ULTIMATE DOWNWARD LOAD CAPACITY OF THE PIER IS BASED ON A MAXIMUM FFL OF 2700mm, FOR FLOOR HEIGHTS ABOVE 2700mm THE PIER CAPACITY MUST BE CHECKED BY YOUR LOCAL ENGINEER. EZUPIER CAN BE SUPPLIED WITH A 2 OR 4 HOLE BASE PLATE. SHS MIN. STEEL GRADE 350MPa TO AS1163. THE CAPACITIES IN THE TABLES ARE CALCULATED USING AUSTRALIAN LOADING CODE AS1170 AND AS4600. IF A HIGHER CAPACITY IS NEEDED USE A "U" PIER HEAD.



NOTE:
 BASE PLATE ORIENTATION IS PARALLEL TO BEARER.

TERMITE INSPECTION POINT AND PIER HEAD PREFERRED ORIENTATION
 TERMITE INSPECTION SATISFIES NCC REQUIREMENTS BY PROVISION OF A CLEAR INSPECTION POINT.

Structural Design Certification By:

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 QLD PRECO N° 7678
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REV.	DESCRIPTION	DRN.	DATE	DESCRIPTION	REVISION
-	-	-	-	EZUPIER ADJUSTABLE "L" PIERHEAD BOXSPAN CONNECTION DETAILS	-

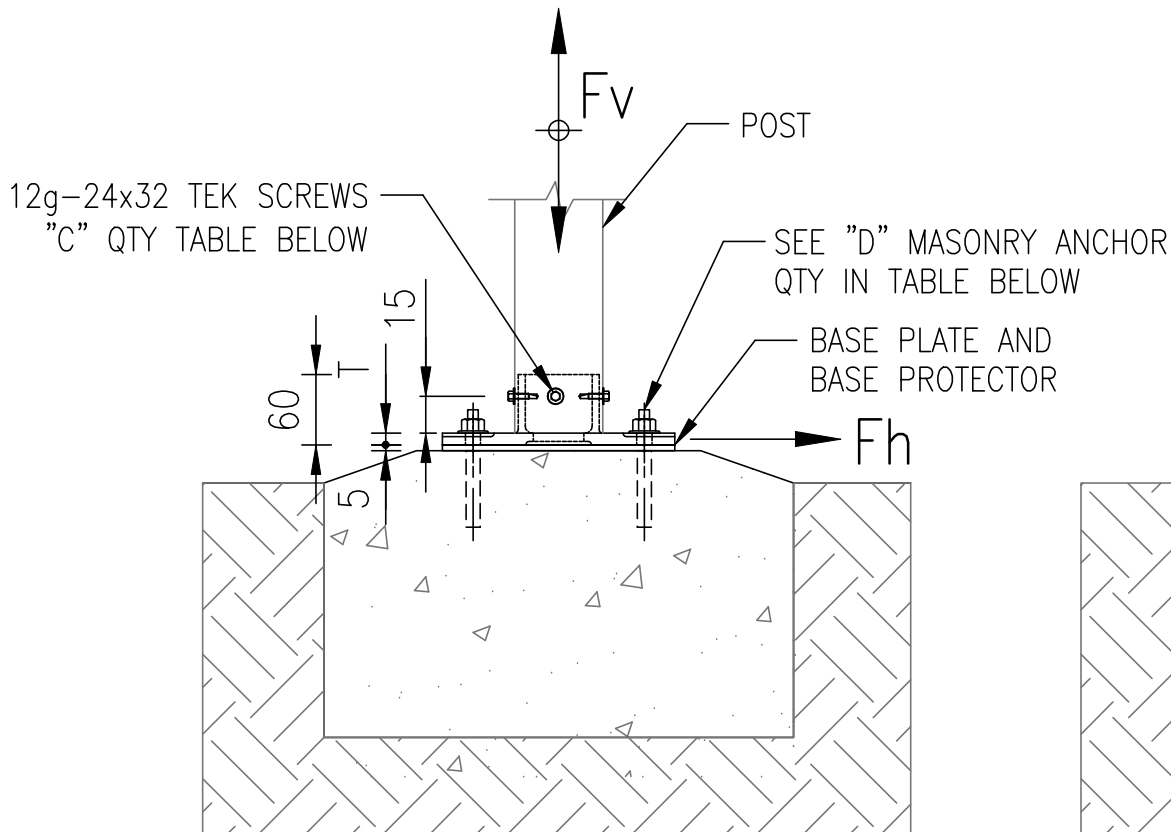
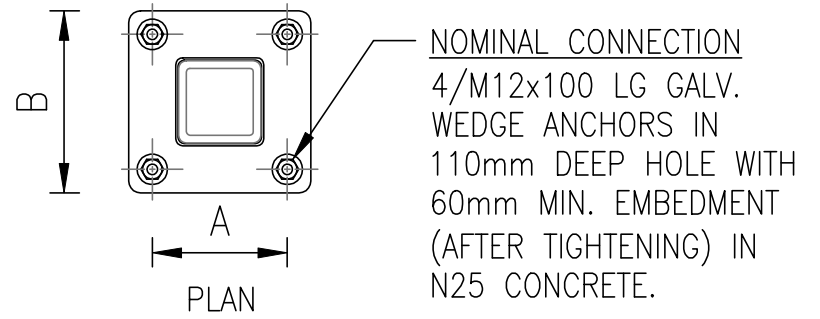
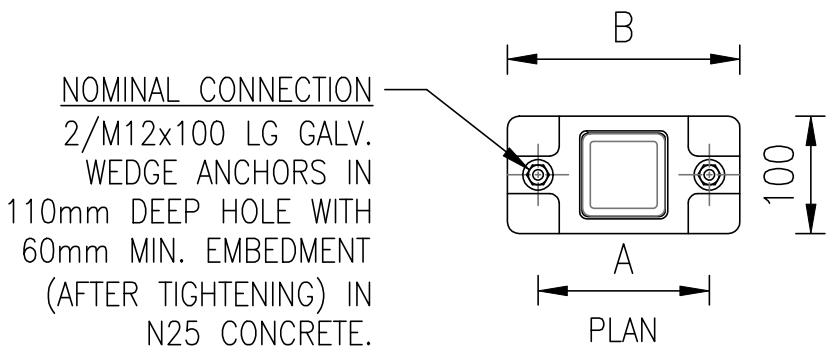
DRAWING NO. **P04-03**

SCALE @ A3

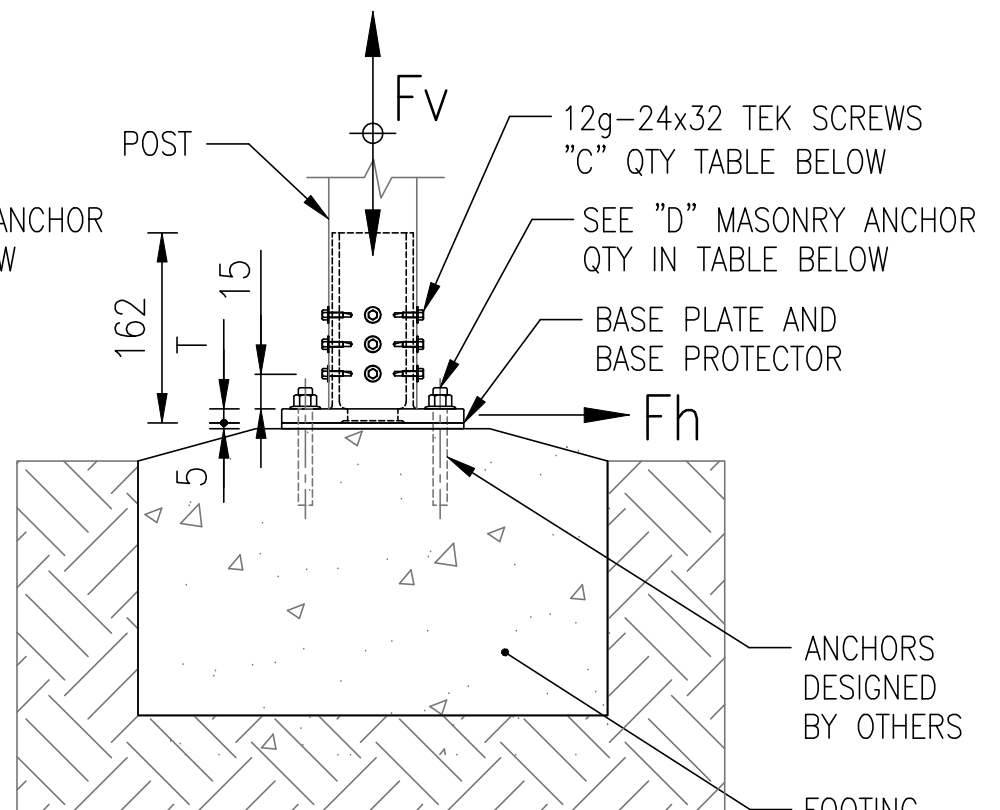
DRAWN AP

DATE DRAWN 29/10/20

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DETAIL .1.
TWO HOLE BASE PLATE
(2HBP)



DETAIL .2.
FOUR HOLE BASE PLATE
(4HBP)

BASE PLATE ULTIMATE CAPACITIES (EXCLUDES ANCHORS)									
BASE PLATE	POST SIZE	Fv Up kN	Fv Down kN	Fh kN	A mm	B mm	T mm	C QTY	D QTY
4HBP	75x2 SHS	69.6	161	101	115	155	12	12	4 *
	90x2 SHS	69.6	195	101	135	190	12	12	
	89x3.5 SHS	70.9	323	101	135	190	12	12	
2HBP	75x2 SHS	19.5	161	42	146	198	10	4	2 *
	90x2 SHS	23.2	182	42	146	198	10	4	
	89x3.5 SHS	22.7	189	42	146	198	10	4	

* THE QUANTITY D FOR THE ANCHORS IS THE NOMINAL FIXING AND NOT THE FIXING FOR THE MAXIMUM UPLIFT.

DESIGN NOTES

- THE TABLE LISTS ULTIMATE LOAD CAPACITIES FOR THE SPANTEC 2 AND 4 HOLE BASE PLATES (EXCLUDES ANCHORS AND FOOTING DESIGN) AND ARE FOR USE IN NORMAL WIND ONLY AND NOT FOR CYCLONIC WIND CONDITIONS. THE 4 HOLE BASE PLATE HAS MAXIMUM UPLIFT LOADS BASED ON THE TEK SCREWS SHOWN. THE TEK SCREWS CAN BE SCALED FOR SMALLER UPLIFT LOADS. THE NOMINAL FIXING SHOWN SHOULD BE USED FOR SMALLER LOADS. ALL LOADS ARE IN KN AND DIMENSIONS ARE IN mm.
- THE FOLLOWING STANDARDS HAVE BEEN USED IN THE CALCULATIONS: AS4100, AS1170.1, AS4055, AS4600, AS3600.
- THE POSTS SHOWN ARE THE MAXIMUM POST SIZE THAT WILL FIT THE INDIVIDUAL BASE PLATES. ALL SQUARE HOLLOW SECTION (SHS) ARE A MINIMUM GRADE OF STEEL G350 TO AS1163.
- THE BASE PLATE IS MADE FROM DUCTILE CAST IRON WITH A MINIMUM ULTIMATE TENSILE STRENGTH OF 400M MPA CONFORMING TO AS1831-2007 (ISO1083) AND HOT DIPPED GALVANISED TO 450 GSM (GRAMS PER SQUARE METER).
- CONCRETE USED IN THE CALCULATIONS IS BASED ON A MIN. COMPRESSIVE STRENGTH F'_c OF 25MPa.
- THE LOADS IN THE TABLES ARE BASED ON USING 12g-24x32 SELF DRILLING TEK SCREWS. THE TABLE SHOWS THE NUMBER OF SCREWS "C" REQUIRED FOR THE MAXIMUM UPLIFT.
- THE TABLES GIVE THE MAXIMUM VERTICAL FORCE DOWN/UP AND MAXIMUM HORIZONTAL FORCE. THE LOADS ARE NOT ALL CONCURRENT I.E. THE MAXIMUM UPLIFT IS NOT AT THE MAXIMUM HORIZONTAL FORCE. THE ACTUAL LOADS SHOULD BE COMBINED AND THE FASTENERS AND MEMBERS RE-CHECKED FOR THE COMBINED FORCES BY THE DESIGN ENGINEER.
- THE MOMENT CAPACITY OF THE BASE PLATE IS NOT STATED. IF THERE ARE MOMENTS ON THE PIERS THEN THE DESIGN ENGINEER SHOULD CONFIRM THE PIER CAN RESIST THE FORCES.
- THE SUPPORTING FOOTING SHOULD BE SIZED BY THE DESIGN ENGINEER BASED ON THE LOADS AND SOIL TYPE ACCORDING TO THE STANDARD AS2870.
- THE BASE PLATE SHOULD BE CONNECTED TO THE FOOTING BY MASONRY ANCHORS SO THAT THE APPLIED MAX. LOAD CAPACITIES CANNOT BE EXCEEDED. ANY ANCHOR USED SHOULD BE GALVANISED AND CAPABLE OF TAKING THE ACTUAL LOADS THE PIER IS SUBJECTED TO. A NOMINAL MASONRY ANCHOR IS SHOWN WHICH IS INDICATIVE ONLY AND NOT THE DESIGN TO TAKE THE MAXIMUM FORCES SHOWN. THE LOCAL ENGINEER SHOULD DESIGN THE MASONRY ANCHORS TO RESIST THE ACTUAL LOADS. THE BASE PLATES ARE NOT DESIGN TO TAKE ANY MOMENTS ON THE PIERS. IT IS ASSUMED THAT THE SUPPORTED FLOOR HAS SUB FLOOR BRACING TO TAKE THE HORIZONTAL WIND. SEE DRAWING P02 & P03 FOR PIER HEAD AND POST DETAILS OR VISIT OUR WEBSITE www.spantec.com.au

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CPEng Chartered Professional Engineer
QLD RPEQ N° 7678
VIC RBP Civil Engineer VBA N° EC24907
TAS Civil Engineer CC5469A

REV.	DESCRIPTION	DRN.	DATE
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DESCRIPTION
EZIPIER
2 AND 4 HOLE BASE PLATES
ULTIMATE CAPACITIES

DRAWING NO.	REVISION
P14	—
SCALE @ A3 NTS	DATE DRAWN 12/08/19
DRAWN AP	

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